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1       **Engineering Properties of Self-Compacting Concrete Incorporating Coal**

2       **Bottom ash (CBA) as Sustainable Materials for Green Concrete: A Review**

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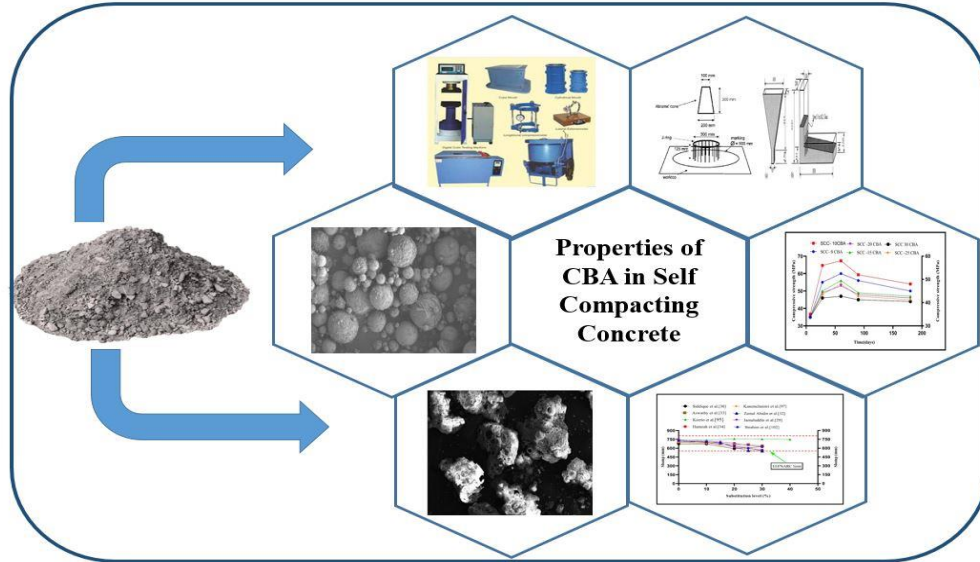
9       **Abstract**

10       Over the past two decades, concrete has been frequently employed in the construction sector  
11       because of its features. The development of massive concrete buildings with more complicated  
12       geometries and dense reinforcing has been growing progressively. Moreover, there is an increased  
13       need for improving the current practices of concrete technology to create new forms of concrete  
14       with better qualities, which encouraged scholars to advance further investigations in this area of  
15       research. Consequently, an innovative type of concrete called Self-Compacting Concrete (SCC)  
16       has been improved. Simultaneously, one key challenge, confronted by the civil engineering sector,  
17       is how to go more environmentally friendly. Using reused waste materials, e.g., coal bottom ash  
18       (CBA), is one of the carefully utilized techniques in construction and building applications. The  
19       CBA's pozzolanic characteristic with high silica and its useful pozzolanic capabilities have  
20       effectively turned CBA into a beneficial substitute in self-compacting concrete. Therefore, CBA  
21       has been successfully employed in producing SCC. Research into CBA function in SCC  
22       production not only contributes to increasing its use but also helps decrease the cost of landfills  
23       and provides a clean, sustainable, and environmental solution by conserving energy and reducing  
24       the depletion of natural resources. In this study, an overview of previous studies on CBA's physical  
25       and chemical characteristics has been thoroughly presented. Moreover, the impact of CBA on the  
26       self-compacting concrete's fresh and mechanical properties is discussed. Results indicated that  
27       using up to 10% CBA in SCC as sand replacement resulted in improved fresh and hardened  
28       properties.

29 **Keywords:** Coal Bottom Ash, Self-Compacting Concrete, Compressive Strength, Green Concrete,  
 30 Mechanical Properties, Sustainable Concrete, Replacement Material, Fresh Properties.

31

32 **Graphic Abstract**



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37 **List of Abbreviations.**

38

- SCC            Self-Compacting Concrete
- CBA            Coal Bottom Ash
- FA             Fly Ash
- EIA            Environmental Impact Assessments
- EDX           Energy Dispersive X-ray Spectroscopy
- W/B            Water to Binder ratio

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47 **1. Introduction**

48 Concrete is a significant component in construction [1]. Concrete has been developed from a  
49 material, which mainly incorporates cement, aggregate, water, in addition to a few admixtures to  
50 a specifically engineered material, which is made of various additional ingredients that enhance  
51 performance in a range of exposure circumstances [2, 3]. Concrete is a frequently utilized building  
52 material due to its beneficial characteristics [4, 5]. However, conventional concrete has some flaws  
53 in the commercial use of ordinary concrete. To tackle several issues of limited construction, such  
54 as intricate designs, the density of reinforcing steel in structural components, a shortage of skilled  
55 labor, the construction industry's fast development, and the poor quality of construction, have  
56 altogether prompted the implementation of self-compacting concrete (SCC) to address these issues  
57 [6–8]. In 1988, Okamura was the first engineer to introduce the utilization of SCC, which was  
58 developed to tackle various problems related to building extremely overcrowded reinforced  
59 concrete components [9–12]. The aim of developing self-compacting concrete primarily involves  
60 creating an innovative concrete type. This concrete can flow through and fill the mold corners and  
61 reinforcing spaces without using vibrations or compacting throughout the casting process [13, 14].  
62 Self-compacting concrete, shortened as (SCC) is defined as a concrete process that uses a  
63 specialized technology for flowing and filling the space in between reinforcing and the high-  
64 performance concrete [15]. Furthermore, this type of concrete, i.e., SCC amalgamates  
65 appropriately under self-weight, and it increases the resistance of hardened concrete without  
66 segregation, bleeding, or any other heterogeneity of components [16–18]. Such concrete qualifies  
67 are useful for practical uses [16–18]. Using plasticizers and powder admixtures is, thus, critical for  
68 the flowability, stability, and impermeability of self-compacting concrete as the hardened concrete  
69 longevity can be enhanced and compressive, tensile, and bending strengths are improved [19].  
70 Sulphates and chlorides exhibit an increase in their concrete resistance. Also, it is eco-friendly and

71 effectively minimizes carbon dioxide emissions and noise generated by dynamic compaction [20,  
72 21]. The efficiency and long-term benefits of using mineral admixtures and additional cementitious  
73 materials like fly ash and Coal Bottom Ash (CBA) are relevant to SCC to achieve durability growth  
74 and maintain a wider economy towards a cost-cutting strategy [22, 23]. Besides, the incorporation  
75 of mineral admixtures allows for the preservation of workability and long-term properties [22, 24].  
76 The growing exploitation of natural resources has resulted in a noticeable increase in industrial  
77 waste and environmental pollution [25, 26]. Therefore, it is essential to minimize the use of natural  
78 resources considerably so that future generations' activities will not be endangered [27–29]. As a  
79 result, it has become increasingly critical to find innovative, alternative materials for sustainable  
80 development. The coal-fired power plant is, however, the main energy-generation source in almost  
81 every country worldwide [30]. Coal thermal power stations have been generating huge FA and  
82 CBA amounts for many years so far, which accounted for 20 to 80%, respectively of total  
83 emissions in the environment [31, 32]. Malaysia contributes to approximately 1.7 mil tons of CBA  
84 and 6.8 mil tons of fly ash production each year. CBA utilization in the building sector is a practical  
85 replacement option for reducing a variety of ecological problems [31, 33]. Previous studies  
86 delivered a list of various management techniques for using CBA. The incipient concerns of CBA  
87 are directly linked to the scarcity of disposal locations, the unrelenting growth of production, and  
88 the continuous loss of natural resources [30, 34]. Previous findings on using CBA as a suitable  
89 construction material provided several advantages to the industry, such as the fact that it is  
90 relatively inexpensive, light, and suitable for utilization as a suitable sand aggregate substitute in  
91 the SCC the application [35–37].

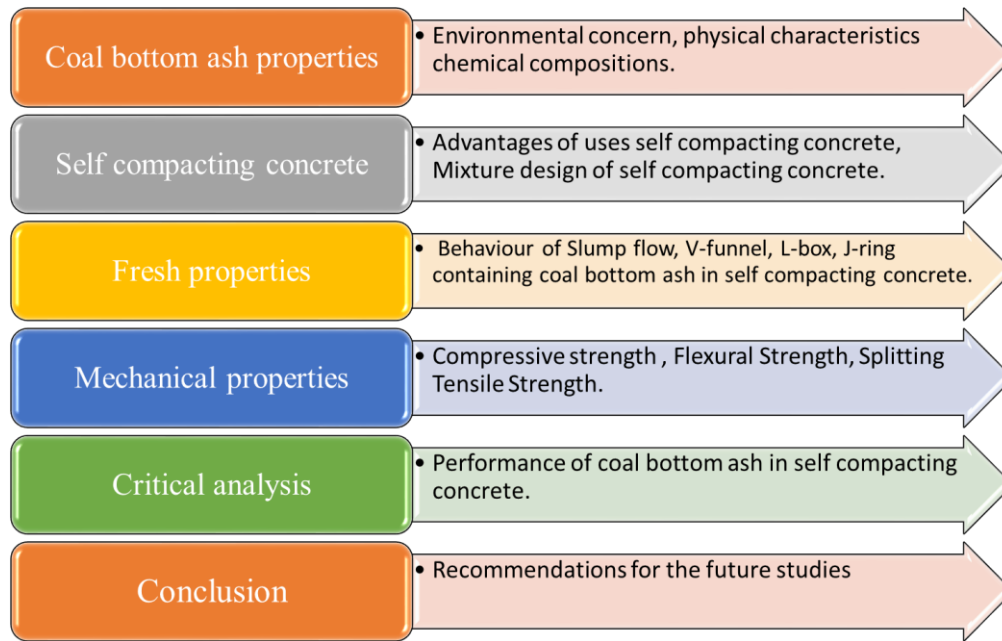
92 The use of CBA, in the current and future construction sector has been promoted because it has a  
93 similar particle size distribution, as well as additional pozzolanic characteristics as a replacement

94 material in SCC. Therefore, it has been widely researched and debated as an alternative material  
95 [38, 39]. According to previous researchers [37, 40–42], SCC is made by mixing CBA with cement  
96 additives, including metakaolin and fly ash. However, with the inclusion of a water reduction  
97 agent, fine aggregate is substituted with 10 to 30 % of CBA. Similarly [43, 44] found that SCC  
98 containing fly ash and CBA has been made to conform to the required criteria of fresh SCC and,  
99 as determined. According to [42] the optimum replacement level of fine aggregate with CBA can  
100 be up to 10% that would provide comparable performance to concrete made with 100% fine  
101 aggregate. Upon employing the mechanical characteristics of the SCC mix as a guide, it was shown  
102 that the optimum proportion of CBA can be replaced in the combination of up to 20% of fine  
103 aggregate. Moreover, studies highlighted CBA’s significant benefits, as the bulk of the SCC  
104 mechanical characteristics produced by adding 10% to 15% of CBA instead of fine aggregate,  
105 enhanced overall performance [41]. For utilizing CBA, the physical and chemical properties,  
106 together with fresh, as well as mechanical properties in the SCC mixture should be investigated.  
107 The review in this paper aims to outline the findings of previous studies about the utilization of  
108 CBA in self-compacting concrete. This paper also aims to provide a critical and comprehensive  
109 review of previous research on the CBA utilization as a suitable alternative material in SCC mixes.  
110 Furthermore, this paper aims to examine the CBA effect on concrete performance.

## 111 **2. Research significance.**

112 In the construction industry, awareness about sustainable development has been growing  
113 considerably in recent years accompanied by the need to mitigate the industry’s environmental  
114 negative impact. A variety of industrial wastes have, therefore, been utilized in construction,  
115 especially in the production of concrete. Different industrial wastes can be exploited as partial or  
116 whole replacements for various concrete components. Using an industrial waste like CBA as a

117 substitute material in making SCC is a good example. Several studies investigated the parameters,  
118 which affect the SCC's mechanical behavior, as well as fresh concrete properties by using  
119 industrial wastes. In construction, resources are being recycled to produce sustainable concrete.  
120 CBA is a problem that affects people's health and the environment as well. Recycling CBA and  
121 replacing it with main components of concrete could help conserve primary resources and reduce  
122 the emission of gases, which unfavorably influence human health. This review paper demonstrates  
123 that the utilization of CBA in producing SCC can help mitigate the depletion of raw materials.  
124 Limited studies that conducted with the aim of providing a comprehensive review analysis of  
125 previous researches regarding the use of CBA in SCC. Therefore, this review presents and  
126 discusses previous studies on using CBA as a substitute material in construction. The present  
127 review also investigates the CBA effects on the SCC fresh and mechanical properties. To this end,  
128 previous findings are examined and evaluated, and the main concrete properties are reviewed and  
129 compared to achieve optimal results. It is believed that providing a conclusions regarding the best  
130 practice of utilizing such material (CBA) in SCC is viable for guiding future researchers.  
131 A Schematic diagram of the procedures used to conduct this review paper is presented in Fig.1.



132

133

**Fig. 1.** Schematic diagram of the procedures used within this review paper.

134 **3. Coal Bottom Ash**

135 **3.1 CBA As Waste Material**

136 CBA is a key source of industrial waste, which is produced by coal thermal power stations. Using

137 CBA can potentially contribute to sustaining the global economy in construction because CBA

138 plays a key role in mitigating the depletion of natural resources [45]. In the whole world, and

139 particularly in the Malaysian context, eliminating CBA constitutes a crucial issue because of an

140 acute shortage in dumping sites, in addition to the rising environmental impacts. CBA’s chemical

141 properties incorporate high silica (SiO<sub>2</sub>), which is the reason why CBA enjoys good pozzolanic

142 properties [32, 46]. Previous studies stated that the use of CBA in producing concrete contributes

143 to enhancing concrete’s durability and its compressive strength. This is because CBA has an

144 increasing pozzolanic reactivity and filler impact. Therefore, CBA constitutes a beneficial

145 supplementary cementitious material in produced concrete [40, 47]. Furthermore, according to

146 what has been mentioned in 2018 by the American Coal Ash Association, CBA has been broadly



147 utilized in the fill and embankment, as well as other construction applications as demonstrated in  
148 the subsequent Table 1 [48].

149 **Table 1.** Utilization of recycled CBA waste in various applications in (tones) [48]

Application	CBA (tons)
Concrete / concrete production. /grout	785,527
Blended cement / raw feeder for clinker	1,622,612
Structural fill / embankment	871,875
Road bases / sub-bases	159,084
Aggregate	10,237

150

### 151 ***3.2 Environmental sustainability and health Assessment of CBA***

152 The CBA environmental sustainability can be periodically assured by conducting environmental  
153 impact assessments (EIA) throughout a product’s life cycle. However, the open disposal of CBA  
154 by coal thermal power plants, as well as many industrial sectors led to creating considerable  
155 ecological contamination, which resulted in major health hazards [49]. From an environmental  
156 perspective, utilizing CBA in the civil engineering application is a feasible solution for the  
157 significant disposal problems of such waste materials [50, 51]. PÖYKIÖ et al. (2016) emphasized  
158 that CBA disposal must be carefully handled either to avoid or mitigate environmental problems  
159 since CBA use is currently limited and at a low percentage [52]. Siddique et al. and Laura et al.  
160 [53, 54] conducted a CBA chemical analysis to determine the percentages of several existing  
161 elements like copper, zinc, barium, arsenic, mercury, and nickel. Several previous studies found  
162 that the existence of excessive amounts of heavy metals can inflict serious damage to living  
163 organisms’ tissues. Accumulating CBA in wide-open areas increases the likelihood of inhaling  
164 dangerous metals, such as copper, zinc, arsenic, barium, nickel, cadmium, mercury, aluminum,  
165 antimony, selenium, chromium, etc. The amount of metals, therefore, increases the risk of  
166 irreversible damage to the respiratory system, the genital system, and the gastrointestinal system,  
167 as well as lungs, kidneys, and raises the risk of birth abnormalities, and can lead to decreasing

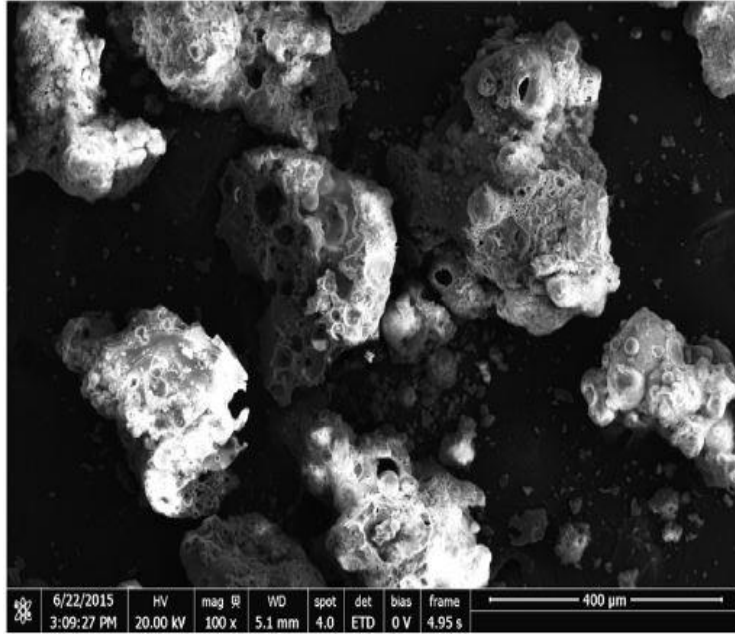
168 bone density in children [55–57]. Furthermore, Hashemi et al. and Aggarwal et al. [58, 59]  
169 examined concrete features of CBA radiation safety. In order of radioactive exposure, using CBA  
170 exerts a more beneficial environmental effect in the concrete industry compared to using it in heaps  
171 and ponds. Fly ash is utilized in the cement industry, and certain facilities have arranged with  
172 power plant operators to collect their fly ash regularly [60, 61]. Therefore, it is essential to research  
173 the CBA chemical composition to determine the risk and behavior associated with its influence on  
174 the environment.

### 175 ***3.3 Physical characteristics and chemical composition of CBA***

176 The CBA physical characteristics and chemical composition in SCC differ not only based on  
177 the coal source but also the process of day-to-day manufacture and the production of coal in coal-  
178 fired power plants. This, in turn, affects the performance of CBA in SCC. The physical  
179 characteristics and chemical composition were investigated as reported by previous studies in the  
180 following sub-section.

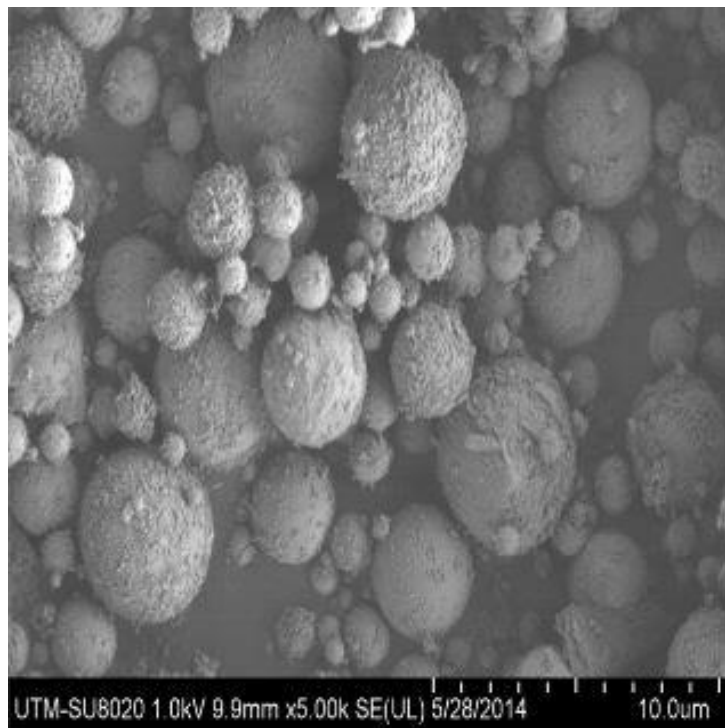
#### 181 ***3.3.1 Physical characteristics***

182 Table 2 provides the CBA’s physical properties according to previous studies. The CBA physical  
183 characteristics in SCC depend on the size of particles and behavior [41, 42]. The color of raw CBA  
184 is grey; however, it becomes rather a dark grey after its grinding process, i.e., blackish [62].  
185 Moreover, following the findings by several researchers, CBA is a substance of a dark grey color  
186 with most of its constituent particles being angular in shape and can be handled as a porous material  
187 of a gruff surface textile of low unit weight. Furthermore, according to the microstructure FESEM  
188 test, various CBA’s are obtained from different sources. As shown in Fig. 2 and Fig. 3, CBA is a  
189 material, which is characterized by a porous irregular shape of a dense structure [45] and [63],  
190 respectively.



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**Fig 2.** Original CBA particles obtained from the Mae Moh thermal power plant [63]



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**Fig 3.** Original CBA particles obtained from Tanjung Bin Power Plant [45]

199 **Table 2.** CBA physical characteristics according to previous studies

References	Specific gravity	Fineness modulus	Water absorptions%
[64]	2.08	1.5	6.8
[65]	2.1	2.93	9.7
[66]	2.08	2.93	9.6
[67]	1.8	-	17
[68]	1.11	2.36	28.85
[69]	2.51	2.77	5.10
[70]	3.83	1.84	6.87
[71]	2.21	2.79	-
[35]	2.08	1.5	6.8
[58]	1.80	-	2.77
[72]	2.1-2.5	1.5	6.8
[73]	1.9	2.4	8.1

200  
201

### 202 **3.3.2 Chemical composition**

203 Scholars have examined the CBA chemical properties, as provided in Table 3 and the chemical  
 204 composition of CBA was studied using microstructure analysis like energy dispersive X-ray  
 205 Spectroscopy (EDX) as mentioned in previous studies [59, 74]. Its chemical composition depends  
 206 on the source of coal, as well as the technique of combustion. CBA has significant concentrations  
 207 of alumina, silica, and iron; these are frequently found in such pozzolanic materials, whose total  
 208 amounts make up between 70 and 93 % of the overall chemical composition of coal. The loss on  
 209 ignition of CBA has been reported by previously conducted studies, ranging between 0.8 and 13.0.  
 210 CBA is categorized as either (Class C or Class F) pozzolanic material, following ASTM C618 [75]  
 211 and following data gathered from a variety of studies. Nonetheless, the CBA coarse particle size  
 212 can lead to a lower degree of pozzolanic reactivity. Therefore, when CBA is ground to smaller  
 213 sizes, this enhances silica and CBA reactivity.

214

215 **Table 3.** CBA’s chemical composition properties based on previous studies

Chemical elements/ References	[76]	[51]	[77]	[78]	[79]	[80]	[81]	[82]	[83]	[84]
SiO <sub>2</sub>	52.4	45.37	59.82	56.44	42.51	48.0	62.32	41.70	58.7	44.10
Al <sub>2</sub> O <sub>3</sub>	27.5	25.12	27.7	26.24	23.52	20.1	27.21	17.10	20.1	9.21
Fe <sub>2</sub> O <sub>3</sub>	6.6	5.81	3.77	8.44	10.2	8.77	3.57	6.63	6.2	24.30
CaO	2.4	0.99	1.86	0.75	12.55	7.11	0.50	22.50	9.5	13.00
MgO	1.83	1.16	0.7	0.4	2.45	3.13	0.95	4.91	1.6	1.88
SO <sub>3</sub>		-	1.39	0.24	-	-	-	0.42	0.4	
K <sub>2</sub> O	3.48	3.87	1.61	1.29	2.12	-	2.58	0.40	1.0	1.25
Na <sub>2</sub> O	0.36	0.64	0.33	0.09	2.2	-	0.70	1.38	0.1	-
TiO <sub>2</sub>	0.97	2.84	-	3.36	0.41	1.11	2.15	3.83	-	-
P <sub>2</sub> O <sub>5</sub>	0.12	0.18	-	-	0.17	-	-		1.0	-
Loss on ignition	3.8	13.1	4.69	0.89	3.82	8.10	-	1.13	0.8	-
SiO <sub>2</sub> +Al <sub>2</sub> O <sub>3</sub> +Fe <sub>2</sub> O <sub>3</sub>	86.5	76.3	91.29	91.12	76.23	76.87	93.10	65.43	85.0	77.61

216

217 **4. Self Compacting Concrete**

218 **4.1 Advantages Use of Self-Compacting Concrete**

219 Recently, a considerable significance has been directed towards SCC due to the benefits it  
 220 provides, such as minimizing building time, lowering labor costs, and improving structural  
 221 compaction for reinforced complicated shapes [14, 85, 86]. The SCC containing CBA has fresh  
 222 characteristics to guarantee that it functions very well in the mixture [64, 87]. The SCC fresh  
 223 characteristics, including mobility, filling capability, passing capability, and segregation  
 224 resistance, are critical criteria that influence the effectiveness of SCC with the CBA use [88, 89].  
 225 In general, SCC can be obtained by utilizing a new superplasticizer generation by reducing the  
 226 ratio of water/binder (w/b in concrete). SCC has one downside, which involves the increased costs

227 due to using chemical admixtures like superplasticizers [42]. Additionally, supplementary material  
228 ingredients, such as CBA are utilized to enhance the SCC viscosity and lower its cost [47].  
229 Moreover, previous scholars examined self-compacting concrete, utilizing various additives,  
230 namely CBA. They found that CBA is an effective ingredient for SCC because of its lightweight  
231 and hard nature in reducing environmental contamination caused by this waste. Furthermore, the  
232 utilization of natural resources linked with main materials production causes severe environmental  
233 impacts [90–92].

#### 234 *4.2 Mixture design of self-compacting concrete*

235 In 1995, H. Okamura introduced the first recommended SCC mixture design, dubbed as the  
236 Japanese or rational technique at that time. SCC is capable of flowing under its weight for filling  
237 gaps in the piece of work, flowing freely all through the reinforcing bars without external vibrator  
238 support. Also, it allows further concrete's accurate leveling when placed without any segregation  
239 [93–95]. Therefore, when the SCC mixture design was developed, various assessments were  
240 conducted, such as V-Funnel, slump flow, and L-Box to evaluate fresh state properties and the  
241 SCC limitation according to EFNARC guidelines as illustrated in Table 4 [96]. This method is  
242 referred to as an empirical design [96]. Nan Suet al. [97] presented the conventionalized empirical  
243 design approach's simple application for implementation and stated that this method may  
244 significantly decrease the amount of time, binders, and expense involved. This approach primarily  
245 aims to enhance workability via the utilization of a specified amount of paste material for keeping  
246 every aggregate material in place, thus increasing the freshness and strength of the concrete. This  
247 is often referred to as the technique of packing aggregates closely [10]. This means that SCC  
248 should use a minimum coarse aggregate amount volume, an increased paste volume, an increased  
249 powder volume, a lowering water-to-powder ratio, a higher superplasticizer dose, and an

250 infrequently necessary viscosity modifying agent [97]. Recent studies have summarized the SCC  
 251 mixture design [98] and, in 2015, it was found that the SCC mixture design is determined by five  
 252 factors. These include 1) an empirical design method, 2) a close aggregate packing technique, 3)  
 253 paste rheology, 4) a method of compressive strength, and 5) a statistical factorial approach [96].  
 254 Previous research has shown that the SCC compressive strength ranged between 20 and 100 MPa  
 255 at 28 curing days, with 40 MPa as a mean compressive strength [99–101]. SCC has a high  
 256 resilience degree. Also, no extraordinary mix design is required if it achieves real applicability,  
 257 economy, practicability, and high quality in both its fresh and hardened states. General exhibits  
 258 are determined as a part by the basic materials used in the mix design [102].

259 **Table 4.** Recommended values of SCC fresh properties following EFNARC [96]

Slump flow		V-funnel test		L-box test	
Slump flow classes	Slump flow (mm)	Viscosity classes	V-funnel times (s)	Passing ability classes	Blocking ratio (H2/H1)
SF1	550–650	VF1	≤8	PA1	≥ 0.8 with 2 bars
SF2	660–750	VF2	9-25	PA2	≤ 0.8 with 3 bars
SF3	760–850	-	-	-	-

260

## 261 **5. Fresh properties of self-compacting concrete incorporating CBA**

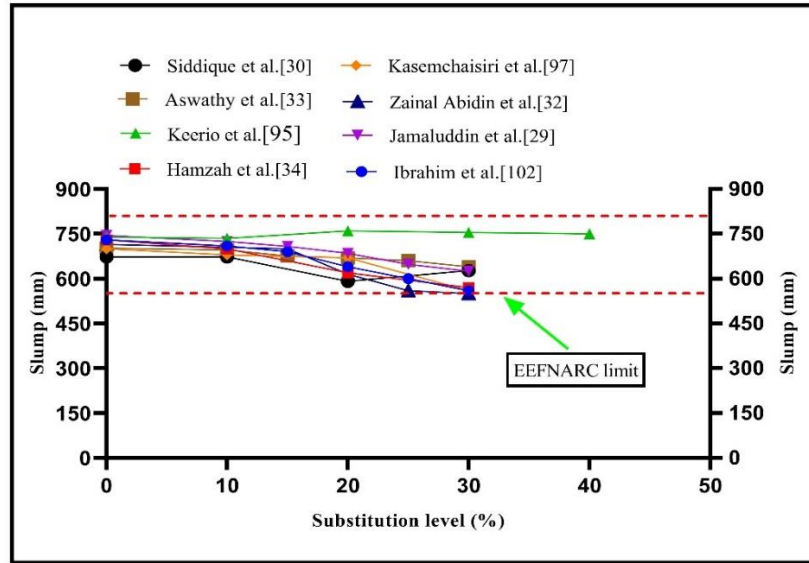
262 As displayed in Table 5, previous studies reported the results of fresh properties tests of CBA in  
 263 the SCC, showing the range of fresh properties and the impact of adding varying ratios of CBA  
 264 substitution in SCC.

### 265 **5.1 Slump Flow Test**

266 The slump flow test involves an average diameter of a concrete volume after releasing the  
 267 conventional slump cone. It can be assessed on a couple of perpendicular sides [96]. As shown in  
 268 Fig. 4, previous research identified the CBA quantity impact on the SCC passing ability. Siddique  
 269 et al. [38] studied the SCC fresh properties with the use of CBA at 10%, 20%, 30%, and this

270 superplasticizer amount (1.88-2.0%). The results showed that slump flow increased for all the  
271 replacement percentages, excluding the one containing 20% CBA and 1.90% of superplasticizer  
272 (591mm) [38], as per the guidelines outlined by the standard EFNARC, ranging between (650-800  
273 mm) of SCC [96]. Increases in the superplasticizer amount resulted in a higher flow, while  
274 increases in the CBA amount resulted in a reduction of slump flow [38]. In their study, Ibrahim et  
275 al. [42] studied the slump flow for SCC and the results revealed a 740-540 mm range and reported  
276 a relative difference between various mixes because the flow characteristics of self SCC mixtures  
277 contain CBA, which have a higher viscosity than control specimens [42]. Another study by Keerio  
278 et al. [103] produced various SCC mixes using CBA (10-40%) as a fine aggregate substitute and  
279 found that CBA content increased and the slump flow decreased due to the CBA porosity, which,  
280 compared with fine aggregate, absorbed more water. The findings of another study, conducted by  
281 Hamzah et al. [104] revealed that the time of slump flow increased with an increase in the CBA  
282 replacement content in SCC. Mixtures containing 0-30% CBA content showed slump flow time,  
283 ranging between 2 and 5 seconds, i.e., in line with the range outlined by EFNARC [96]. The reason  
284 is that the CBA's irregular shape lowered inter-particle friction with the addition of CBA, which  
285 decreased the SCC mixes' viscosity.





286

287

**Fig 4.** Variation of slump values in SCC with varying CBA substitution ratios.

### 288 5.2 L-Box Test

289 The percentage of L-box elevation can be used in tandem with the H2/H1 ratio to define the self-

290 compacting concrete's passing abilities [96]. Several previous studies examined the effect of CBA

291 quantity on the SCC L-box test, as shown in Fig.5. Kasemchaisiri et al. [105] reported that the L-

292 box passage ratio was between 0.83 and 0.05 and that the value decreased in all mixtures as the

293 CBA percentage in SCC increased. A reduction in the L-box ratio due to the presence of aggregate

294 blockage in a mixture of self-compacting concrete, containing CBA, was detected and a greater

295 level of inter-particle friction was produced by the CBA particles [105]. Similarly, another study

296 by Jamaluddin et al. [37] reported a decrease in the L-box test results with increasing the

297 replacement proportion of CBA incorporated into the SCC mixture. Nevertheless, when the w/b

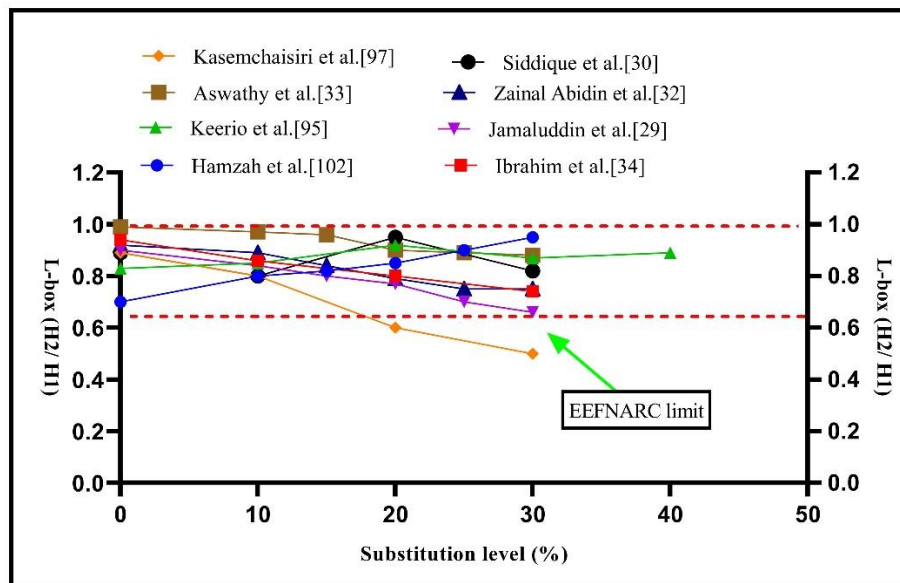
298 proportion increases, the value of the L-box decreases. The results varied between 0.9 and 0.66,

299 indicating that the concrete mixture without CBA had a higher passing ability [37]. This is due to

300 the aggregate clogging up in front of apertures, which made it difficult for particles to flow freely

301 without occlusion [37].

302 Hamzah et al. [104] observed that the L-box flow results improved with increasing the CBA  
 303 proportion in concrete. The range is between 0.7 and 0.95 [104]. According to Singh et al. [35], an  
 304 increase was observed in the L-box ratio, thereby increasing the replacement proportion of CBA  
 305 in the SCC mixture. The value reached up to 1.0 in a 30% of CBA, with a greater value, which  
 306 showed an improved passing ability [35]. EFNARC standard guideline [96] categorized L-box  
 307 passing ratios as (passing ability 1 (PA1) with two rebars) or (passing ability 2 (PA2) with three  
 308 rebars). The findings provided by each researcher indicated that the L-box passing ratio is within  
 309 the acceptable range [96].



310  
 311 **Fig 5.** L-box values' variation in SCC using varying CBA replacement ratios

### 312 5.3 V-Funnel Test

313 This conducted test verifies the SCC mixtures' viscosity and flowability. A mixture's better  
 314 flowability denotes a very short flow time and, as per EFNARC, the V-funnel test's duration ranges  
 315 between 6 and 12 seconds [96]. It was established in another previous study that the V-funnel flow  
 316 time for several types of SCC ranged between 2.5-4.6 seconds according to Keerio et al. [103].  
 317 They found that the V-funnel flow time of SCC increases as the replacement ratio of CBA

318 increases. They also confirmed that a high quantity of superplasticizer is required for producing  
319 SCC when CBA is combined with metakaolin, as compared to SCC made without any replacement  
320 materials [103]. Likewise, Aswathy et al. [41] observed an increment in the V-funnel value as the  
321 percentage content of CBA increased. The finding of V-funnel value between 8 and 12 seconds,  
322 signifying an enhanced passing ability [41]. The EFNARC standard guideline [96] categorized V-  
323 funnel flow time into two categories: VF1 (8 s) and VS2 (9–25 s) [96]. According to the findings  
324 obtained by each researcher, the V-funnel flow time of certain results falls into the VF1 category,  
325 while others fall into the VF2 category [96].

#### 326 ***5.4 J-ring Test***

327 The J-ring test, in parallel to the L-box test, can be applied in conjunction with the test of slump  
328 flow to ascertain that concrete can pass-through bars, as per EFNARC [96]. According to ASTM  
329 C1621 [106], J-ring can be defined as a method of testing concrete's capacity of passing through  
330 under its weight, thereby filling spaces, and getting a blocking evaluation [106]. The J-ring flow  
331 results were reported by Siddique et al. [38] which ranged between 2.3 and 11.6 mm for various  
332 self-compacting concrete. The results of the J-ring flow of SCC containing CBA rose as the CBA  
333 amount increased for all the mixtures [38]. Also, based on another study by Keerio et al. [103] the  
334 values of the J-ring ranged between 2.5-8.7 mm. As observed, the values increased with an increase  
335 in the CBA replacement ratios in the SCC mixture [103].

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Table 5. Summary of CBA Fresh properties in the SCC mixtures

Study	CBA replacement (%)	w/b	Superplasticizer content (%)	Fresh concrete properties			
				Slump flow (mm)	L-box (H2/H1)	V-funnel (s)	J-ring h2-h1 mm
[96]	—	—	—	650–800	0.8–1	—	—
[42]	0	0.4	0.16 - 0.36	730	0.94	—	—
	10 %			710	0.859	—	—
	20 %			640	0.80	—	—
	30 %			570	0.74	—	—
[104]	0	0.35, 0.40, 0.45	0.16-0.30	730	0.70	—	—
	10%			710	0.8	—	—
	15%			690	0.82	—	—
	20%			640	0.85	—	—
	25%			600	0.9	—	—
	30%			560	0.95	—	—
. [103]	0	0.38	2.0 - 17.0	740	0.83	7.4	2.5
	10%			735	0.85	11.2	4.5
	20%			760	0.92	11.89	7.4
	30%			755	0.87	10.22	8.7
	40%			750	0.89	12	6.8
. [37]	0	0.35, 0.40, 0.45	0.16-0.32	745	0.90	—	—
	10%			725	0.81	—	—
	15%			708	0.80	—	—
	20%			685	0.77	—	—
	25%			648	0.70	—	—
	30%			625	0.66	—	—
[105]	0	0.31	1,200 cc/m.3	700	0.83	—	—
	10%			680	0.80	—	—
	20%			670	0.60	—	—
	30%			560	0.50	—	—
[38]	0	0.41-0.55	1.88 - 2.0	673	0.89	7.50	2.3
	10			673	0.80	6.60	4.6
	20			591	0.95	6.20	4.7
	30			627	0.82	4.0	11.6
[41]	0	0.68	5.16	702	0.99	8	—
	5%			696	0.97	8	—
	10%			676	0.96	10	—
	15%			670	0.9	11	—
	20%			660	0.89	12	—
	30%			640	0.88	12	—
[40]	0	0.4	0.20	715	0.92	—	—
	5%			705	0.89	—	—
	10%			700	0.84	—	—
	15%			615	0.79	—	—
	20%			560	0.75	—	—
	30%			550	0.65	—	—

344

345 **6. Discussion of finding from fresh Properties**  
346

347 Existing research indicates that the ideal content of CBA is between 10% and 20% to meet the  
348 criteria of "filling and passing ability" and "segregation resistance". The dependence of the result  
349 depends on the water-binder ratio and the amount of superplasticizer used. An increase in the CBA  
350 content has been observed to have a negative effect on processability and segregation. CBA has  
351 the potential to be used at 10% to 20% in SCC, meeting standards set by the European Federation  
352 of National Associations Representative for Concrete (EFNARC). Based on the above-mentioned  
353 previous studies, it was observed that the key properties of CBA in SCC, namely slump and L-box  
354 ratio, show a decrease, while J-ring and V-funnel values increase with increasing substitution ratio  
355 of CBA in the concrete mix. The reduction in the freshness properties of the mixture can be  
356 attributed to the absorption of moisture content by the CBA particles, resulting in increased friction  
357 between the aggregate particles generated by the CBA particles. The conclusions from all of the  
358 previous studies support the criteria and limitations established in the SCC standards and  
359 recommendations, including the requirements of EFNARC, 2002, and ACI 237R-07.

360 **7. Mechanical properties of self-compacting concrete incorporating CBA**

361 The next sub-sections discuss the mechanical properties of self-compacting concrete (SCC)  
362 incorporating CBA. The discussion includes compressive strength, flexural tensile strength, and  
363 splitting tensile strength. Table 6 presents a summary of the CBA effect on the SCC mechanical  
364 properties based on previous studies.

365 **6.1 Compressive strength**

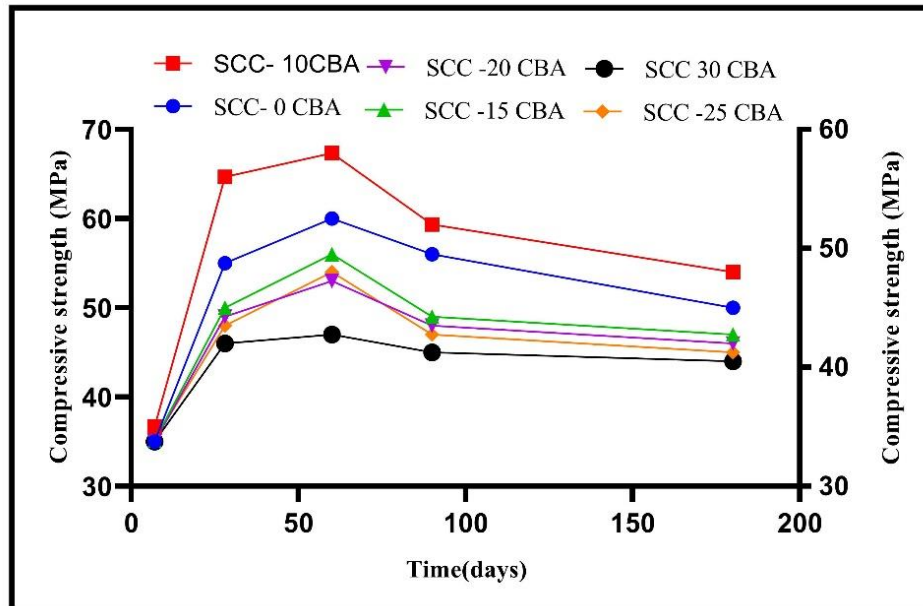
366 Compressive strength along with fresh characteristics are regarded as the significant valuable  
367 features of self-compacting concrete (SCC), providing a complete picture of the condition of  
368 concrete as they are closely associated with the cement paste structure [6]. Furthermore, the  
369 concrete's compressive strength at room temperature is influenced by ambient curing, water-to-  
370 cement ratio, aggregate particles size and category, aggregate-paste interface transition zone,  
371 categories of admixture, and stress applied [107]. Previous researchers [47, 105, 108] confirmed  
372 the CBA suitable percentages of use in the SCC mix, i.e., 10%-20%; such a percentage improved  
373 compressive strength properties. Siddique et al. [44] examined how the compressive strength of

374 the SCC containing CBA and coal fly ash can be affected by the utilized water-to-powder ratio.  
375 The results showed a comparable behavior with regular SCC when strength was increased by  
376 lowering the w/p ratio. Self-compacting concrete showed enhanced strength properties as the w/p  
377 ratio decreased, i.e., 0.439-0.414 for the 0% CBA, a decrease from 0.50 to 0.47 for the 10% CBA,  
378 with a decrease from 0.58 to 0.51 for the 20% CBA, and from 0.620 to 0.546 for the 30% CBA.  
379 Self-compacting concrete can, therefore, be produced at 40-50 MPa compressive strength using  
380 various percentages of CBA merged with coal fly ash between 15% and 30%.

381 As reported by Kasemchaisiri [105] the SCC compressive strength with CBA is 10%-30%,  
382 compared with the control samples without CBA. It was found that 10 % of CBA obtained higher  
383 values of compressive strength. Other percentages were decreased, which can be attributed to  
384 delayed pozzolanic reaction, which dominated over the raised porosity. Similar observations were  
385 also made by [108], whereby increasing replacement amounts of CBA ranging from 10% to 30%  
386 decreased the compressive strength. The decrease was between 54 MPa and 42 MPa at 180 curing  
387 days, as demonstrated in Fig. 6. Another study by Zainal Abidin et al. [40] recorded an increased  
388 compressive strength up to 15% CBA in the SCC mix. These increased properties of strength  
389 revealed that there was a pozzolanic reactivity in SCC containing CBA particles. Compressive  
390 strength obtained these increases: 44.30MPa, 50.33MPa, 54.05MPa, 37.90MPa, 36.65MPa for  
391 CBA0%, CBA10%, CBA15%, and CBA20%, CBA25%, respectively.

392 A reduction in concrete strength was obtained by Siddique et al. [38] throughout the curing initial  
393 stages due to CBA's delayed pozzolanic reactivity. However, the concrete strength improved  
394 significantly during the long period of curing. The compressive strength in SCC decreased with  
395 increasing the water-to-binder ratio and the proportion of CBA replacement. According to the  
396 conducted review of previous studies, the behavior of SCC containing CBA can be affected by the

397 percentages of CBA, the superplasticizer inclusion, and the utilized w/c ratio. Therefore, adding a  
398 finer size of CBA using appropriate proportions encourages the formation of pozzolanic reactions.  
399

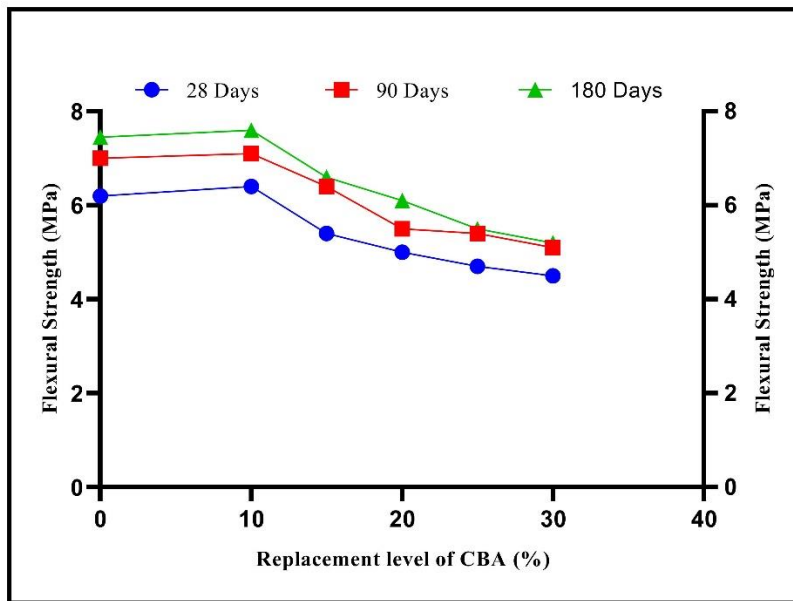


400  
401 **Fig 6.** The samples' compressive strength with varying replacement percentages in SCC [108]

## 402 **6.2 Flexural strength**

403 According to Jamaluddin et al. [37], a reduced flexural strength for SCC incorporating CBA was  
404 observed at different ages, as exhibited in Fig.7. Fine aggregate was substituted with CBA at  
405 varying ratios of replacement up to 30%. Furthermore, the optimal percentage of 10% CBA is  
406 higher when compared with control samples. The authors reported an increased flexural strength  
407 with a higher value of 8.2 MPa with 0.35 w/c during the lengthy time of curing ages. Conversely,  
408 the flexural strength was decreased for all water-cement ratios when 15%, 20%, 25%, and 30%  
409 alternative of CBA is utilized in SCC [37]. Another study by Zainal Abidin et al. [91] reported  
410 that CBA's flexural strength in self-compacting concrete with 10% and 15% alternative of CBA  
411 is greater compared to SCC without the addition of CBA at 7-28 ages of curing. Furthermore,  
412 flexural strength for other replacement percentages was decreased.

413 Another study by Siddique et al. [44] reported that the flexural strength of SCC was decreased as  
414 CBA increased. They showed that when the CBA replacement ratio increases, flexural strength  
415 drops because of a weaker contact between the cement paste and ashes. As per Keerio et al. [103],  
416 the SCC mixes' flexural strength with CBA from (10% to 30%) decreased as the replacement  
417 percentages increased at various ages starting from 3 to 180 days of curing in contrast to regular  
418 concrete without CBA. The reduction occurred due to low inter-particle abrasion between the  
419 aggregate particles, as the CBA particles have spheres formed.



420  
421 **Fig 7.** Flexural strength of samples with various replacement ratios in SCC at different curing  
422 ages [37]

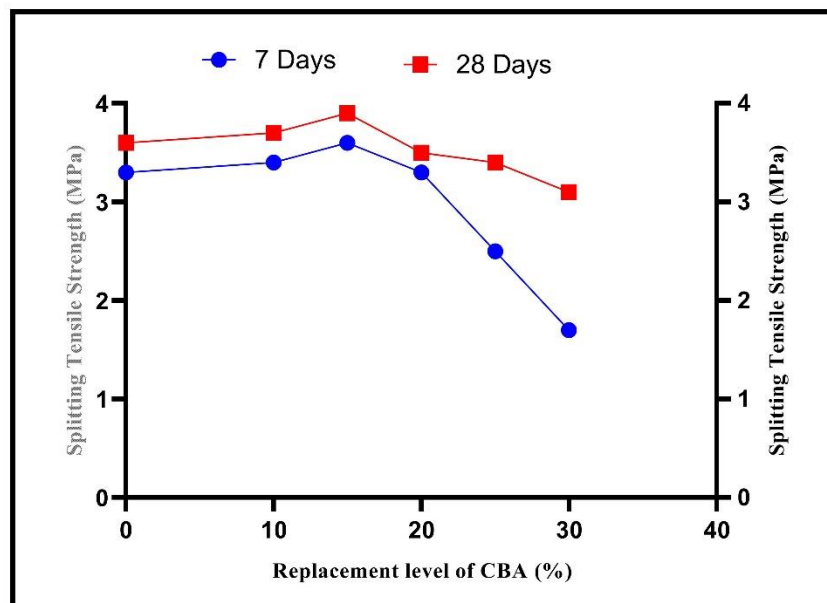
### 423 **6.3 Splitting tensile strength**

424 Various mechanical characteristics of concrete can be assessed using one of the fundamental and  
425 significant characteristics [42]. The splitting tensile strength of the concrete structures significantly  
426 affects the cracking development and size [42]. Concrete is weak under tension and, therefore, it  
427 is essential to conduct a preliminary assessment of the concrete's splitting tensile strength [42].  
428 Another study by Siddique et al. [38] examined the splitting tensile strength of self-compacting



429 concrete containing 10%, 15%, 20%, 25%, 30% of CBA as a partial replacement for fine aggregate  
 430 in concrete. There was a decrease in the concrete's splitting tensile strength with increased amounts  
 431 of CBA at the entire ages of curing due to insufficient interlocking of the fine aggregate particles  
 432 replacing the fine aggregate with CBA. Similar trend observations were also made by Sandhya et  
 433 al. [109], who reported a decrease in the SCC splitting tensile strength with increased CBA  
 434 replacement percentages, while strength increased with curing ages.

435 Another study by Zainal Abidin et al. [91] recorded splitting tensile strengths of 3.6, 3.75, 3.9, 3.5,  
 436 3.4, and 3.1 MPa at 28 days, in the examined SCC mixture containing CBA 0,10%, 15%, 20%,  
 437 25%, 30%, respectively. An increase was reported in the splitting tensile strength of the samples  
 438 as the CBA replacement ratio reached 15%, as shown in Fig.8. Similarly, another study by  
 439 Aswathy et al. [41] reported an increase in the splitting tensile strength as the replacement ratio of  
 440 CBA reached 10% using a fixed 0.45 w/c ratio. This increased CBA replacement resulted in more  
 441 porous concrete, having larger pores, which were scattered across the CBA aggregate surface,  
 442 thereby decreasing its tensile strength [41].



443

444 **Fig 8.** CBA samples' splitting tensile strength with varying replacement percentages in SCC at  
 445 different curing ages [91]

446

447

Table 6. Summary of mechanical properties of CBA on SCC mixes

Reference	CBA replacement ratio (%)	Finding		
		Compressive Strength	Flexural Strength	Split Tensile strength
[44]	0, 10%, 20%, 30%	Decreased for all replacement percentages	-	Decreased for all replacement percentages
[62]	0, 10%, 20%, 30%	Reduction for all replacement percentages	-	Decreased for all replacement percentages
[64]	0, 10% CBA mixed with 25%, 50%, 75% and 100% recycled coarse aggregate	Decreased for all mixes and all replacement percentages for early age for long term increased in each percentage.	-	Decreased for all mixes and all replacement percentages for early age for long term increased in each percentage.
[35]	0, 10% of CBA mixed with 20% and 30% of FA, and 10% of MK	Increased in all percentages of CBA replacement with RCA up to 50 after that drop in higher percentages	-	A drop in all percentages of replacement expect at 50% of RCA
[38]	0, 10%, 20, 30%	Decreased in all replacement percentages at all curing ages	-	Decreased in all replacement percentages at all curing ages
[37]	0,10%,15%,20%,25%,30%	-	Increased by 10% replacement other percentages was decreased as CBA increased	-
[40]	0,10%,15%,20%,25%,30%	Increased by up to 15% CBA and other percentages was decreased as CBA increased.	-	-
[105]	0 %, 10 %, 20 % and 30 %	increased by 10% CBA and other	-	-

		percentages were decreased as CBA increased		
[47]	10%, 15%, 20%, 25% and 30%	Reduction for all replacement percentages	-	-
[41]	5%, 10%, 15%, 20%, 25% and 30%	Increased by 5% and 10 %, while other percentages were decreased as CBA increased	Increased by 5% and 10%, while the other percentages were decreased as CBA increased.	Increased by 5% and other percentages were decreased as CBA increased.
[110]	0,10%,15%,20%,25%,30%	Increased by up to 10% CBA and other percentages was decreased as CBA increased.	Increased by up to 10% CBA and other percentages were decreased as CBA increased.	Increased by up to 10% CBA and other percentages were decreased as CBA increased.
[91]	0,10%,15%,20%,25%,30%	Increased by up to 15%, and other percentages were decreased as CBA increased.	Increased by up to 15%, and other percentages were decreased as CBA increased.	Increased by up to 15%, and other percentages were decreased as CBA increased.
[103]	0.10%,20%,30%,40%	Increased in all replacement percentages	Increased in all replacement percentages	Increased in all replacement percentages
[111]	10 % CBA, 30 % FA and 25% to 100 % RCA.	Increased by 10 % CBA with RCA up 50%, and other percentages of RCA with CBA were decreased.	-	-
[108]	0,10%,15%,20%,25%,30%	Increased by 10% and other replacement percentages were decreased as CBA increased	-	-

448

449 **8. Discussion of finding from Mechanical Properties**

450 The investigation of the influence of the CBA volume in SCC on its strength characteristics  
451 revealed that the incorporation of CBA as a replacement material at levels of up to 10% yielded a  
452 significant improvement relative to traditional concrete. Previous research has shown that the  
453 incorporation of a modest amount of CBA in SCC has resulted in notable improvements in

454 mechanical parameters, including compressive strength, split tensile strength, and flexural  
455 strength. These enhancements have seen to reach up to 10% in the majority of the results. The  
456 increase in strength may be ascribed to the pore refinement effect resulting from the pozzolanic  
457 activity of CBA. The use of CBA as a replacement material leads to an enhanced level of porosity.  
458 However, it is worth noting that the presence of silica in CBA particles plays a crucial role in  
459 facilitating the synthesis of calcium-silicate-hydrate (C-S-H), a gel-like substance that  
460 significantly contributes to the development of strength in materials. The observed increase in  
461 strength may be attributed to the higher concentration of calcium silicate hydrate (C-S-H) in the  
462 SCC samples mixed with CBA. This increase in C-S-H is a consequence of the reaction between  
463 the calcium hydroxide produced during cement hydration and the reactive silica present in the  
464 CBA.

## 465 **Conclusion**

466 The aim of this research was to provide a comprehensive review regarding the use of CBA in SCC.

467 Based on the conducted review the following conclusions were drawn:

- 468 • CBA can be used to replace up to 20% of the fine aggregate in SCC without negatively  
469 affecting its fresh properties and up to 10% without reducing its mechanical properties  
470 considerably.
- 471 • Extending the age of curing for samples with CBA can significantly enhance the properties  
472 of SCC due to the delayed pozzolanic reaction of CBA.
- 473 • The CBA's physical and chemical characteristics are suitable to be effectively utilized in  
474 producing self-compacting concrete.
- 475 • The CBA inclusion affects fresh properties like (L-box, V-funnel, slump flow, J-ring) in  
476 the SCC mixture. The L-box height ratio ranged from (0.8 to 1.0), which is in the range of  
477 the EFNARC standard for all SCC mixtures as mentioned by several researchers. For other  
478 fresh properties like (V-funnel, slump flow, J-ring) in self-compacting concrete, they were  
479 decreased as the substitution ratio of CBA increased.

## 480 **Future Recommendations**

481 The utilization of SCC in the building industry has been growing steadily and according to  
482 previous studies, SCC has several appealing features. It contributes to enhancing fresh and  
483 hardened characteristics, which makes it perfect for effective use in the construction sector. By  
484 adding CBA and chemical admixtures, as well as a variety of aggregate percentages, improved  
485 fresh and hardened characteristics were obtained. The evaluation has been conducted in line with  
486 this review and sufficient materials were provided. However, many gaps remain in this field of  
487 research, which should be explored further. To concentrate on the construction industry's use of  
488 SCC, it is, therefore, recommended to examine the effect of CBA inclusion on the durability  
489 characteristics, such as bond strength, impact resistance, and abrasion resistance. Apart from these  
490 mechanical characteristics, time-dependent mechanical characteristics of SCC, such as creep and  
491 shrinkage are suggested topics for further studies to narrow the scope of research. The inclusion  
492 of CBA as a pozzolanic material and chemical admixtures (superplasticizers) increases the  
493 capacity of SCC to pass through and flow. Additionally, limited research has been conducted on  
494 the rheological and thixotropic behavior of SCC. Therefore, to obtain a better understanding of  
495 workability, such behaviors should be investigated by incorporating CBA into SCC.

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## 502 **Authors' Contribution**

503 **Mohammad I. Al Biajawi** Writing- Original draft preparation, Writing Reviewing and Editing.  
504 **Rahimah Embong:** Supervision, Writing- Original draft preparation, Writing Reviewing and  
505 Editing.  
506 **Ali Shubbar** Writing Reviewing and Editing.

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#### 514 **Data Availability**

515 Data will be made available on request.

#### 516 **Declarations**

#### 517 **Competing Interests**

518 The authors declare no competing interests.

#### 519 **Human and Animal Rights and Informed Consent**

520 This article does not contain any studies involving animals or human participants performed by  
521 any of the authors.

#### 522 **Ethics Approval**

523 We confirm the ethic approval.

#### 524 **Consent to Participate**

525 All the authors and contributors have consent for this article.

#### 526 **Consent for Publication**

527 All authors and contributors have consent to publish in this journal.

#### 528 **Conflicts of Interest/Competing Interests**

529 There are no conflicts/competing of interests.

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